Stability Analysis of Geology Structures Controlled Tunnel Profiles

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Location of Jurong Rock Cavern Project, Singapore





Birds eye view of Jurong Rock Cavern Project, Singapore

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Geology of JRC project showing sub-horizontal bedded structures of Ayer Chawan Facies





Mudstone interbedded with pyroclastic rock (Lithic Tuff) and sandstone



- Traditionally circular, arch and horse shoe shaped tunnel profiles are designed
- Smooth stress distribution after excavation for better stability condition

Bergen hill tunnel: source – Google images : horse shoe shaped tunnel



Flat-roofed tunnels / caverns in JRC project were excavated in sub-horizontal bedded sedimentary, Ayer Chawan facies of Jurong Formation







The shaded rock mass became unstable in arch-shaped roof cavern excavated in horizontal bedded rocks with vertical joints (Pells, Crockford and Diederichs)



a. Vertical and horizontal jointed rock mass dissociated and deflects in the crown of an excavation

b. A compression arch forms to prevent the deflection of two discrete blocks (a Voussoir beam analogue)

(Diederichs & Kaiser)

1)

2)

11)

12)

13)

14)

- Left Flow chart for the determination of stability and deflection of a Voussoir beam (Diederichs & Kaiser, 1999)
- Right Stability analysis results of **storage cavern** following L flow chart
- Individual bed thickness = 0.4m
- Dip angle of bed $\neq 10$ degree
- Cavern span/s = 20 m
- Unit weight, $r = 27 \text{ kN/m}^3$
- $UCS, \sigma = 164.5 \text{ MPa}$



At the available lowest N value = 0.4, F.S (crushing) = 3.42F.S (sliding) = 6.58Displacement (mid-span deflection/bed thickness) ratio = 36.8%. Buckling limit, (1 - N) = 0.6.

The above displacement ratio and buckling limit are close to the collapse limit proposed by Diederichs & Kaiser.

Caverns are stable after tunnel supports (80 mm thick shotcrete with 5 m long rock bolts) were applied

Analytical approach

Intact Rock Properties in Phase² Analysis

General Intact Rock Properties							
Young's modulus	65000 MPa	Material Type	Plastic				
Poisson's ratio	0.3	Dilation Angle	0				
Failure Criterion	Mohr-Coulomb	Residual Friction Angle	32.4 deg				
Tensile strength	0 MPa	Residual Cohesion	8.4 MPa				
Peak Friction angle	10.3 deg	Initial Element Loading	Field Stress				
Peak Cohesion	68.6 MPa						

General Joints Properties						
Properties	Joint Set 1 (Bedding plane)	Joint Set 2	Joint Set 3			
Normal stiffness (MPa/m)	10000	10000	10000			
Shear stiffness (MPa/m)	1000	1000	1000			
Cohesion (MPa)	0.3	0.2	0.2			
Friction (degree)	30	30	30			
Initial joint deformation	Allowed					
Slip criteria	Mohr-Coulomb					
Tensile strength (MPa)	0					

Tunnel Support properties

Shotcrete Properties						
Liner Type	Formulation	Thickness	Young's Modulus	Poisson's ratio		
Standard Beam	Timoshenko	0.08m	20000 MPa	0.15		

GFRP Rock Bolt Properties (Glass Fibre Reinforced Polymer Bolts)

Bolt Type	Plain Strand Cable	Water : cement	0.3
Borehole diameter	44mm	Out-of-plane spacing	2.2m
Cable diameter	22mm	Face Plate	Attached
Young's modulus	50000 MPa	Allow joints to shear Bolt	Yes
Cable Peak strength	0.35 MN		

Phase² FEM analysis of storage cavern with staged excavations and tunnel support application





2 D model of storage cavern (20 m W, 340 m L, 27 m H)

Calculated vertical displacements (meter) at cavern crown after final excavation and tunnel support (80 mm thick shotcrete with 5 m long GFRP bolts at 2.2 m spacing)





Location of convergent monitoring points for displacement monitoring inside cavern

Vertical displacement monitoring results

Vertical displacement monitoring results are comparable with those obtained by Phase² analysis

CONCLUSIONS

Two main controlling factors for stability of compression arch (Voussoir beam)

Sufficient horizontal stress and
Sufficient layer thickness

In Jurong Rock Cavern Project

1. $S_H : S_h ; S_v = 2.2 : 1.8 : 1.0$ (high horizontal stress ratio) 2. Individual sedimentary layers are of 0.35 m minimum thickness

