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## Shakedown analysis of lined rock cavern for compressed air energy storage

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## **Background - CAES**



- Compressed air energy storage (CAES) systems
- Lined or unlined rock caverns
- Gas pressure: 10 to 30 MPa
- Repeated compression-decompression cycles



#### **Background – carven behavior**



#### Creeping effect is not considered

## Background

- The cavern may exhibit plastic strain during each loading cycle as yielding occurs in loading and unloading processes.
- **Reverse yielding** can lead to **low-cycle fatigue** of elastic-plastic materials. E.g. failure will occur in steel specimen after 1000 cycles for a strain range of around 0.02 (Stephens, 1980).
- Johansson (2003) suggested to check low-cycle fatigue of steel lining of lined rock cavern.
- Low-cycle fatigue needs to be considered in the survivability analysis of rock openings subjected to multiple attack (Balachandra, 1978).
- **Shakedown analysis** can provide the maximum capacity of a structure against reverse yielding under cyclic loads based on elastic-plastic theory.
- Melan's shakedown theorem:  $f(\sigma_e + \sigma_r) \leq Y$
- For the CAES carven problem, if the stress state after unloading is smaller than the yield criterion, shakedown will occur; otherwise, reverse yielding will happen.

## An analytical approach – problem definition

- Two layers of isotropic homogenous linear elastic cohesive-frictional materials (Mohr-Coulomb criterion)
- Combine both cylindrical scenario (k=1) and spherical scenario (k=2)
- Reverse yielding occurs or not in both materials when fully





## An analytical approach

• Yield or not?

$$\alpha \sigma_r - \sigma_\theta = Y$$

• When loaded: (Mo et al., 2014)

$$\alpha_A \sigma_{r,A} - \sigma_{\theta,A} = Y_A$$
$$\alpha_B \sigma_{r,B} - \sigma_{\theta,B} = Y_B$$

• When unloaded:

$$\alpha_A \sigma_{r,A}^{\prime\prime} - \sigma_{\theta,A}^{\prime\prime} = Y_A$$
$$\alpha_B \sigma_{r,B}^{\prime\prime} - \sigma_{\theta,B}^{\prime\prime} = Y_B$$

in which:

$$\sigma_{r,A}^{\prime\prime} = \sigma_{r,A} + \Delta \sigma_{r,A}$$
$$\sigma_{\theta,A}^{\prime\prime} = \sigma_{\theta,A} + \Delta \sigma_{\theta,A}$$
$$f(P_a, r) ?$$

$$\alpha = \frac{1 + \sin\phi}{1 - \sin\phi}$$
$$Y = \frac{2C\cos\phi}{1 - \sin\phi}$$

$$\alpha_n = \frac{1 + \sin\phi_n}{1 - \sin\phi_n}$$
$$Y_n = \frac{2C_n \cos\phi_n}{1 - \sin\phi_n}$$
where *n* = *A* or *B*

$$\sigma_{r,B}^{\prime\prime} = \sigma_{r,B} + \Delta \sigma_{r,B}$$
$$\sigma_{\theta,B}^{\prime\prime} = \sigma_{\theta,B} + \Delta \sigma_{\theta,B}$$
$$f(P_h, r) ?$$

 $\sim f(P_a, r) \\ \sim f(P_b, r)$ 





• Yield or not?

$$\alpha \sigma_r - \sigma_\theta = Y$$

• When loaded: (Mo et al., 2014)

 $\alpha_A \sigma_{r,A} - \sigma_{\theta,A} = Y_A$  $\alpha_B \sigma_{r,B} - \sigma_{\theta,B} = Y_B$ 



(u) Donaling

 $\sim f(P_a, r)$  $\sim f(P_b, r)$ 

• When unloaded:

$$\alpha_A \sigma_{r,A}^{\prime\prime} - \sigma_{\theta,A}^{\prime\prime} = Y_A$$
$$\alpha_B \sigma_{r,B}^{\prime\prime} - \sigma_{\theta,B}^{\prime\prime} = Y_B$$

in which:

$$\sigma_{r,A}^{\prime\prime} = \sigma_{r,A} + \Delta \sigma_{r,A}$$
$$\sigma_{\theta,A}^{\prime\prime} = \sigma_{\theta,A} + \Delta \sigma_{\theta,A}$$
$$f(P_a, r) ?$$

$$\sigma_{r,B}^{\prime\prime} = \sigma_{r,B} + \Delta \sigma_{r,B}$$
$$\sigma_{\theta,B}^{\prime\prime} = \sigma_{\theta,B} + \Delta \sigma_{\theta,B}$$
$$f(P_h, r)$$
?



• When unloaded:

in

which:  

$$\begin{array}{c}
\alpha_{A}\sigma_{r,A}^{\prime\prime} - \sigma_{\theta,A}^{\prime\prime} = Y_{A} \\
\alpha_{B}\sigma_{r,B}^{\prime\prime} - \sigma_{\theta,B}^{\prime\prime} = Y_{B} \\
\end{array}$$

$$\begin{array}{c}
\sigma_{r,A}^{\prime\prime} = \sigma_{r,A} + \Delta\sigma_{r,A} \\
\sigma_{\theta,A}^{\prime\prime} = \sigma_{\theta,A} + \Delta\sigma_{\theta,A} \\
f(P_{a},r) f(P_{a},r) f(P_{a},r) \\
\end{array}$$

$$\begin{array}{c}
r = a \\
P_{a,SA} \\
\sigma_{a,SA}^{\prime\prime} = P_{a,SA} \\
\sigma_{r,B}^{\prime\prime} = \sigma_{r,B} + \Delta\sigma_{r,B} \\
\sigma_{\theta,B}^{\prime\prime} = \sigma_{\theta,B} + \Delta\sigma_{\theta,B} \\
f(P_{b},r) \\
\end{array}$$





• When unloaded:

in

which:  

$$\begin{array}{c}
\alpha_{A}\sigma_{r,A}^{\prime\prime} - \sigma_{\theta,A}^{\prime\prime} = Y_{A} \\
\alpha_{B}\sigma_{r,B}^{\prime\prime} - \sigma_{\theta,B}^{\prime\prime} = Y_{B} \\
\end{array}$$

$$\begin{array}{c}
\sigma_{r,A}^{\prime\prime} = \sigma_{r,A} + \Delta\sigma_{r,A} \\
\sigma_{\theta,A}^{\prime\prime} = \sigma_{\theta,A} + \Delta\sigma_{\theta,A} \\
f(P_{a}, r) f(P_{a}, r) f(P_{a}) \\
\end{array}$$

$$\begin{array}{c}
r = a \\
P_{a,SA} \\
r = a \\
P_{a,SA} \\
\sigma_{r,B}^{\prime\prime} = \sigma_{r,B} + \Delta\sigma_{r,B} \\
\sigma_{\theta,B}^{\prime\prime} = \sigma_{\theta,B} + \Delta\sigma_{\theta,B} \\
f(P_{b}, r) \end{array}$$



• When unloaded:

in which:  

$$\begin{array}{c}
\alpha_{A}\sigma_{r,A}^{\prime\prime} - \sigma_{\theta,A}^{\prime\prime} = Y_{A} & r = a \\
\alpha_{B}\sigma_{r,B}^{\prime\prime} - \sigma_{\theta,B}^{\prime\prime} = Y_{B} & r = b \\
\alpha_{B}\sigma_{r,B}^{\prime\prime} - \sigma_{\theta,B}^{\prime\prime} = Y_{B} & r = b \\
\alpha_{B}\sigma_{r,B}^{\prime\prime} - \sigma_{\theta,B}^{\prime\prime} = Y_{B} & r = b \\
\sigma_{r,A}^{\prime\prime} = \sigma_{r,A} + \Delta\sigma_{r,A} & Assume P_{b}^{\prime\prime} = P_{0} & \sigma_{r,B}^{\prime\prime} = \sigma_{r,B} + \Delta\sigma_{r,B} \\
\sigma_{\theta,A}^{\prime\prime} = \sigma_{\theta,A} + \Delta\sigma_{\theta,A} & \sigma_{\theta,B}^{\prime\prime} = \sigma_{\theta,B} + \Delta\sigma_{\theta,B} \\
f(P_{a},r) f(P_{a},r) f(P_{a}) & f(P_{a},r) f(P_{b},r) f(P_{a},r)
\end{array}$$

#### **Results – numerical validation (cylindrical case)**



#### **Results – numerical validation (spherical case)**



- P<sub>0</sub> = 2.5MPa (around 100m depth)
- Rock friction angle  $\phi$  = 37°
- Rock unconfined compression strength  $\sigma_c$  = 10MPa
- Rock cohesion =  $\sigma_c(1 sin\phi)/(2cos\phi) = \sigma_c/4$
- Rock tensile strength =  $\sigma_c/3$
- Rock Young's Modulus = 10GPa
- Rock Poisson's ratio = 0.3
- Steel yield strength = 355MPa
- Steel Young's Modulus = 198GPA
- Steel Poisson's ratio = 0.32
- Cavern radius = 2m



# Shakedown limit for a spherical rock cavern with steel lining

## **Concluding remarks**

- Analytical solutions for shakedown conditions of a cavity in two layers of cohesivefrictional materials are developed.
- It has been proved by numerical simulations that the proposed analytical solutions provide admissible cyclic loading conditions for the cavity to avoid reverse yielding.
- The shakedown limit pressure of underground lined rock caverns for the CAES systems, could then be determined by using the developed solutions, which can avoid low-cycle fatigue.
- Further work will be conducted considering concrete lining and better estimation of  $P_b''$ .







